Report for
City of Camas Traffic Impact Fee Update


Prepared for
City of Camas

Prepared by
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May 2012

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## ACKNOWLEDGMENTS

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Phil Bourquin, Community Development Director
Eric Levison, Public Works Director
Agency Coordination Committee:
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## CHAPTER 1: EXISTING AND FUTURE BASELINE CONDITIONS ANALYSIS

This chapter introduces the existing and future motor vehicle conditions that will be used to update the Camas Traffic Impact Fee (TIF). The Camas TIF was last updated in 2003. Over the past eight years, the urban growth areas for Camas and other neighboring Cities have expanded and therefore the needs for roadway and intersection improvements have changed. A key element of the TIF update is to identify the areas impacted by the projected growth and determine the associated transportation facility improvements needed to accommodate it.

Existing motor vehicle facility conditions were reviewed to identify deficiencies before the traffic volume growth associated with new development was added to the roadway network in Camas. This ensures that the updated TIF can associate costs with a nexus to development impacts. The existing motor vehicle inventory data also represents the baseline to which future growth in the City will be added, and will be used to help ensure that acceptable operations of roadways and intersections is maintained as new development increases traffic volumes. The following sections provide a summary of the study area, a description of the existing motor vehicle facilities, and an inventory of existing traffic volumes and congestion levels at key intersections in the study area.

## Study Area

The study area is comprised of the Camas urban growth area (or Urban Growth Boundary), which includes the entire Camas City limits, in addition to land just outside or adjacent to the City limits that is planned for future annexation and urbanization.

Figure 1 shows the major roadways in Camas, as well as key study area intersections that were reviewed for motor vehicle intersection operations. The study intersections included:

1. $6^{\text {th }}$ Avenue/Norwood Street
2. $6^{\text {th }}$ Avenue/Ivy Street
3. Division Street/ $6^{\text {th }}$ Avenue
4. Adams Street/6 ${ }^{\text {th }}$ Avenue
5. Dallas Street/SR-500 (3rd Avenue)
6. SR-14/SR-500 (Union Street)
7. $3^{\text {rd }}$ Avenue $/ 2^{\text {nd }}$ Avenue- $4{ }^{\text {th }}$ Street
8. $3^{\text {rd }}$ Avenue/Crown Road
9. $6^{\text {th }}$ Avenue/SR-500 (Garfield Street)
10. $14^{\text {th }}$ Avenue/SR-500 (Everett Street)
11. $18^{\text {th }}$ Avenue/Division Street
12. $28^{\text {th }}$ Avenue/Sierra Drive
13. $18^{\text {th }}$ Avenue/Cascade Street
14. McIntosh Road/Brady Road
15. $16^{\text {th }}$ Avenue/Brady Road
16. Pacific Rim Boulevard/Payne Road
17. Pacific Rim Boulevard/Parker Street
18. $38^{\text {th }}$ Avenue/Parker Street
19. Lake Road/Sierra Street
20. Lake Road/SR-500 (Everett Street)
21. $43^{\text {rd }}$ Avenue/SR-500 (Everett Street)
22. Leadbetter Road/SR-500 (Everett Street)
23. Nourse Road-15 ${ }^{\text {th }}$ Street $/ 283^{\text {rd }}$ Avenue
24. Lake Road/Parker Street
25. Lake Road $/ 218^{\text {th }}$ Avenue
26. $1^{\text {st }}$ Street/Friberg Street-202 ${ }^{\text {nd }}$ Avenue
27. $13^{\text {th }}$ Street/Friberg Street
28. Goodwin Road/Camas Meadows Drive
29. Goodwin Road/Ingle Road
30. $28^{\text {th }}$ Street $/ 232^{\text {nd }}$ Avenue


## Motor Vehicle Facilities

Characteristics of the major roadways in the urban growth area of Camas were documented and are presented in Figure 1. Data collected included functional classification, roadway crosssection, and posted speed limits.
State Route (SR) 14 and SR 500 are the state highways in Camas. SR 14 is classified by the state as a Highway of Statewide Significance (HSS) ${ }^{1}$, while SR 500 is classified by the state as a Regionally Significant Highway. SR 14 runs east to west and connects the City of Camas to I205, the City of Vancouver, other nearby urban areas to the west, and the Columbia River Gorge to the east. SR 500 generally winds north to south through Camas via the alignments of several roadways, connecting SR 14 at the south to $28^{\text {th }}$ Street at the north.
Major roadways under City of Camas jurisdiction include Brady Road, Parker Street, Pacific Rim Boulevard, SE $20^{\text {th }}$ Street/NW $38^{\text {th }}$ Avenue, NW $16^{\text {th }} / \mathrm{Hood} / 18^{\text {th }}, 1^{\text {st }}$ Street, Lake Road, Dallas Street (between $3^{\text {rd }}$ and $6^{\text {th }}$ ), $3^{\text {rd }}$ Avenue and $6^{\text {th }}$ Avenue. Each of these roadways are classified as arterials ${ }^{2}$ and generally provide for higher volumes of motor vehicle circulation through the City.

## Completed TIF Roadway Improvements

A few of the improvement projects included in the 2003 Camas TIF have been constructed. These projects mitigated forecasted roadway deficiencies that resulted from new growth in Camas. The completed projects include:

- Leadbetter Road: Constructing a new two lane roadway from Parker Street to Lake Road.
- $1^{\text {st }}$ Street/Lake Road: Widening $1^{\text {st }}$ Street and Lake Road to three or five lanes.


## Existing Traffic Volumes

Motor vehicle activity at 30 intersections in the study area was collected during the weekday evening peak hour (4:00 p.m. to 6:00 p.m.) in the late spring and early summer of 2011. In addition, historical motor vehicle count data from recent years (2007 to 2010) for 10 intersections was obtained ${ }^{3}$ and utilized to supplement the new count data. The count data was used to analyze existing intersection operations at the study intersections, and is included in the appendix. The existing evening peak hour traffic volumes developed for the study intersections are displayed in Figure 2.

[^0]| 1. th Ave/Norwood St. | 2. 6 th Ave./Ivy 5 st. | 3. Division St/./th Ave. | 4. Adams St./Gth Ave. | 5. Dallas St/spr. 500 (3rd Ave.) | 6. sR-14/SR-500 (Union St.) | 7. 3rd Ave./2nd Ave. (4th St.) | 8. 3rd Ave./Crown Rd. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 9. 6 th Ave./SR. 500 (Garfield $S$ t.) | 10. 14th Ave./SR.500 (Everett $S$ t.) | 11. 18th Ave./Division St. | 12. 28 it Ave. Sierra 0 or | 13. 18 th Ave./Cascade St. | 14. Melintosh Rd.Brady R | 15. 16 th Ave. Brady Rd. | 16. Pacific Rim Blvd/Payne Rd. |
| 17. Pacific Rim Elvd./Parker St. | 18. 38th Ave.Parker St. | 19. Lake Rd/Sierra St. | 20. Lathe Roadss.50 (Everert St.) | 21. 43 rr Ave./SR. 500 (Everett $S t$.) |  |  |  |
| 22. Leadbetter Rd/SR. 500 (Everett Rad, | 23. Nourse Rd.-15th St:283rdd Ave. | 24. Lake Rd./Parker St | 25. Late Rd./218th Ave. Pagne St.) | 26. 1st St:tribiorg St:202nd Ave. |  |  |  |
| 27. 13th St/friberg St | 28. Goodwin Rd/Camas Meadows Dr. | 29. Goodwin Rd/Ingle Rd. | 30. 28th St/232nd Ave. |  |  |  |  |
|  |  |  |  | LEGEND $\begin{aligned} & \text { (10 - Study Intersection } \\ & \text { - Stop Sign } \\ & \text { - Traffic Signal } \end{aligned}$ |  |  |  |

## Intersection Operations

This section covers the existing traffic operating conditions at the study intersections. Included is a description of the intersection performance measures, jurisdictional operational standards, and an existing traffic operational analysis.
Intersection Performance Measures
Level of service (LOS) and volume-to-capacity (V/C) ratios are two commonly used performance measures that provide a gauge of intersection operations. In addition, they are often incorporated into agency mobility standards. Descriptions are given below:

- Level of service (LOS): A "report card" rating (A through F) based on the average delay experienced by vehicles at the intersection. LOS A, B, and C indicate conditions where traffic moves without significant delays over periods of peak hour travel demand. LOS D and E are progressively worse operating conditions. LOS F represents conditions where average vehicle delay has become excessive and demand has exceeded capacity. This condition is typically evident in long queues and delays.
- Volume-to-capacity (V/C) ratio: A decimal representation (between 0.00 and 1.00 ) of the proportion of capacity that is being used (i.e., the saturation) at a turn movement, approach leg, or intersection. It is determined by dividing the peak hour traffic volume by the hourly capacity of a given intersection or movement. A lower ratio indicates smooth operations and minimal delays. As the ratio approaches 1.00, congestion increases and performance is reduced. If the ratio is greater than 1.00 , the turn movement, approach leg, or intersection is oversaturated and usually results in excessive queues and long delays.


## Jurisdictional Mobility Standards

The mobility standards for the study intersections vary according to the agency of jurisdiction for each roadway. Of the 30 study intersections, seven are under state jurisdiction (including intersections along SR 14 and SR 500), two are under county jurisdiction (Nourse Road-15 ${ }^{\text {th }}$ Street $/ 283^{\text {rd }}$ Avenue and $28^{\text {th }}$ Street $/ 232^{\text {nd }}$ Avenue), while the remaining intersections are under the jurisdiction of the City of Camas.
The Washington State Department of Transportation (WSDOT) requires a level of service "D" or better for Highways of Statewide Significance (HSS) in urban areas ${ }^{4}$, including SR 14. In addition, WSDOT requires a level of service "E" or better for Regionally Significant State Highways (non-HSS) in urban areas, including SR 500. Clark County requires a level of service "E" or better for unsignalized intersections, unless signal warrants are met, then a level of service "D" would be required. ${ }^{5}$ The City of Camas operating standards require that a level of service " D " and a volume to capacity ratio of 0.90 or better to be maintained for all intersections. ${ }^{6}$

[^1]
## Existing Operating Conditions

The existing motor vehicle operating conditions at the study intersections were determined for the evening peak hour based on the 2000 Highway Capacity Manual methodology ${ }^{7}$ for signalized and unsignalized intersections. The conditions include the estimated average delay, level of service (LOS), and volume-to-capacity ( $\mathrm{v} / \mathrm{c}$ ) ratio of the study intersections and are shown in Table $1 .{ }^{8}$

Table 1: Existing Evening Peak Hour Intersection Operations

| Intersection | $\begin{array}{l}\text { Mobility Standard* } \\ \text { LOS }\end{array}$ |  | V/C | Delay |
| :--- | :--- | :--- | :--- | :--- | \(\left.\begin{array}{c}Level of <br>


Service\end{array}\right]\)| Volume/ |
| :--- |
| Capacity |

Signalized Intersections

| Dallas Street/SR-500 (3rd Avenue) | E |  | 9.7 | A | 0.61 |
| :--- | :---: | :---: | :---: | :---: | :---: |
| SR-14/SR-500 (Union Street) | D |  | 30.2 | C | 0.92 |
| $3^{\text {rd }}$ Avenue/2 $2^{\text {nd }}$ Avenue-4 ${ }^{\text {th }}$ Street | D | 0.90 | 5.5 | A | 0.31 |
| $3^{\text {rd }}$ Avenue/Crown Road | D | 0.90 | 9.9 | A | 0.39 |
| $38^{\text {th }}$ Avenue/Parker Street | D | 0.90 | 15.1 | B | 0.41 |
| Lake Road/SR-500 (Everett Street) | E |  | 13.6 | B | 0.49 |
| $43^{\text {rd }}$ Avenue/SR-500 (Everett Street) | E |  | 9.5 | A | 0.37 |
| Lake Road/Parker Street | D | 0.90 | 13.7 | B | 0.51 |
| $1^{\text {st }}$ Street/Friberg Street-202 |  |  |  |  |  |
| $13^{\text {th }}$ Street/Friberg Street | D | 0.90 | 8.4 | A | 0.35 |

All-Way Stop Intersections

| $28^{\text {th }}$ Avenue/Sierra Drive | D | 0.90 | 8.6 | A | 0.24 |
| :--- | :---: | :---: | :---: | :---: | :---: |
| $16^{\text {th }}$ Avenue/Brady Road | D | 0.90 | 13.3 | B | 0.54 |
| Pacific Rim Boulevard/Parker Street** | D | 0.90 | 10.8 | B | 0.46 |

Unsignalized Intersections

| $6^{\text {th }}$ Avenue/Norwood Street | D | 0.90 | 53.6 | A/F | 0.65 |
| :--- | :---: | :---: | :---: | :---: | :---: |
| $6^{\text {th }}$ Avenue/Ivy Street | D | 0.90 | 33.6 | A/D | 0.28 |
| Division Street/ $6^{\text {th }}$ Avenue | D | 0.90 | 19.2 | A/C | 0.30 |
| Adams Street $/ 6^{\text {th }}$ Avenue** | D | 0.90 | 15.4 | A/C | 0.37 |
| $6^{\text {th }}$ Avenue/SR-500 (Garfield Street) | E |  | 16.9 | A/C | 0.26 |

${ }^{7} 2000$ Highway Capacity Manual, Transportation Research Board, Washington DC, 2000.
${ }^{8}$ Detailed intersection analysis worksheets are attached in the technical appendix.

| Intersection | Mobility Standard* <br> LOS |  | Delay | Level of <br> Service | Volume/ <br> Capacity |
| :--- | :---: | :---: | :---: | :---: | :---: |
| $14^{\text {th }}$ Avenue/SR-500 (Everett Street) | E |  | 67.1 | $\mathrm{~A} / \mathrm{F}$ | 0.75 |
| $18^{\text {th }}$ Avenue/Division Street | D | 0.90 | 10.1 | $\mathrm{~A} / \mathrm{B}$ | 0.17 |
| $18^{\text {th }}$ Avenue/Cascade Street | D | 0.90 | 9.3 | $\mathrm{~A} / \mathrm{A}$ | 0.13 |
| McIntosh Road/Brady Road | D | 0.90 | 16 | $\mathrm{~A} / \mathrm{C}$ | 0.29 |
| Pacific Rim Boulevard/Payne Road | D | 0.90 | 15.6 | $\mathrm{~A} / \mathrm{C}$ | 0.33 |
| Lake Road/Sierra Street | D | 0.90 | 12.5 | $\mathrm{~A} / \mathrm{B}$ | 0.28 |
| Leadbetter Road/SR-500 (Everett Street) | E |  | 9.7 | $\mathrm{~A} / \mathrm{A}$ | 0.17 |
| Nourse Road-15 ${ }^{\text {th }}$ Street/283 ${ }^{\text {rd }}$ Avenue | E |  | 9.3 | $\mathrm{~A} / \mathrm{A}$ | 0.08 |
| Lake Road/218 ${ }^{\text {th }}$ Avenue/Payne Street | D | 0.90 | 17.8 | $\mathrm{~A} / \mathrm{C}$ | 0.22 |
| Goodwin Road/Camas Meadows Drive | D | 0.90 | 13.7 | $\mathrm{~A} / \mathrm{C}$ | 0.22 |
| Goodwin Road/Ingle Road | D | 0.90 | 17.1 | $\mathrm{~A} / \mathrm{C}$ | 0.37 |
| $28^{\text {th }}$ Street/232 ${ }^{\text {nd }}$ Avenue | E |  | 14.7 | $\mathrm{~A} / \mathrm{B}$ | 0.17 |

Note:
*Mobility Standard is for City of Camas, except for SR-14, which is WSDOT HSS, SR-500, which is WSDOT Non HSS, and Nourse Road- $15^{\text {th }}$ Street $/ 283^{\text {rd }}$ Avenue and $28^{\text {th }}$ Street $/ 232^{\text {nd }}$ Avenue, which is for Clark County.
**Intersection configuration not allowed in HCM analysis, therefore intersection configuration was modified in Synchro to allow for capacity analysis
Bolded and Shaded indicates mobility standard is not met

Signalized or AWS intersections: All Movements
LOS = Level of Service of Intersection Delay = Average Delay of Intersection V/C = Volume-to-Capacity Ratio of Intersection (except for AWS where V/C is for worst movement)

Unsignalized intersection: Worst Movement
LOS = Level of Service of Major Street/Minor Street
Delay $=$ Approach Delay of Worst Movement
V/C = Volume-to-Capacity Ratio of Worst Movement

During the evening peak hour, all study intersections operate within jurisdictional standards, with the exception of the $6^{\text {th }}$ Avenue/Norwood Street and the $14^{\text {th }}$ Avenue/SR 500-Everett Street intersections. The $6^{\text {th }}$ Avenue/Norwood Street intersection operates at level of service of " $F$ " on the minor street approach due to the high traffic volumes on $6^{\text {th }}$ Avenue causing long delays for northbound traffic on Norwood Street waiting to find an acceptable gap to turn left onto $6^{\text {th }}$ Avenue.
At the $14^{\text {th }}$ Avenue/SR 500-Everett Street intersection, the eastbound approach operates at level of service of "F" due to high traffic volumes from the uncontrolled southbound movement (SR 500-Everett Street) preventing traffic from $14^{\text {th }}$ Avenue to finding an acceptable gap to turn left onto SR 500-Everett Street.

## Signal Warrants

A signal warrant analysis was performed for the unsignalized study intersections not meeting mobility standards to determine if side-street volumes are high enough to justify (i.e. warrant) the construction of a traffic signal. The only unsignalized intersections not meeting mobility standards under existing conditions were the 6th Avenue/Norwood Street and 14th Avenue/SR500 (Everett Street) intersections. For this analysis, the MUTCD9 Warrant \#3 (peak hour) was assessed using 2011 p.m. peak hour traffic volumes. Based on the peak hour warrant, neither of these intersections would meet the signal warrant criteria. The signal warrant analysis worksheets are attached in the appendix.

## 2005 Base Link Volumes

To help understand the traffic flows and corridor conditions throughout the entire study area, the regional travel demand model developed by the Southwest Washington Regional Transportation Council (SWWRTC) was customized for use in Camas. Roadway link data, including estimated volumes and approximate levels of congestion, can be plotted from the model for sketch-level purposes.
Figure 3 shows 2005 model link volumes with links having volume-to-capacity ratios over 0.80 colored to indicate the relative level of congestion. In addition, approximate intersection level of service is indicated as well. This figure does not represent Highway Capacity Manual calculations, but gives a general indication of the performance of the network.
Based on Figure 3, the worst congestion occurs along SR $14,6{ }^{\text {th }}$ Avenue and $1^{\text {st }}$ Street/Lake Road.

[^2]

## Future Base Conditions

The need for transportation improvements within Camas depends on the level of future development and the corresponding traffic volumes. The 2003 Camas Traffic Impact Fee was based on a 2023 traffic forecast. This TIF update uses a 2035 land use forecast to assess future traffic growth. A detailed mesoscopic transportation forecast model was developed for the study area from the Southwest Washington Regional Transportation Council's (RTC) regional travel demand forecast models (base year 2005 and future year 2035) to assess the growth in traffic. The projected growth in traffic was then added to existing volumes to determine traffic volumes for the forecast year 2035. This chapter provides a general description of the forecast methodology and summarizes future roadway operations resulting from the growth in traffic. More detailed information about the forecasting methodology can be found in the Focus-Area Mesoscopic Forecasting Methodology memorandum, in the appendix. ${ }^{10}$

## Future Demand and Land Use

The City of Camas TIF addresses additional facilities that are required to serve future growth. The RTC urban area transportation forecast model was used to determine traffic growth and future volumes in Camas. This forecast model translates land uses into person travel, selects modes, and assigns motor vehicles to the roadway network. These traffic volume projections form the basis for identifying potential roadway deficiencies and for evaluating alternative circulation improvements. This section describes the forecasting process, including key assumptions and the land use scenario developed from the existing Comprehensive Plan designations and allowed densities.

## Projected Land Uses

Land use is a key factor in developing a functional transportation system. The amount of land that is planned to be developed, the type of land uses, and how the land uses are mixed together have a direct relationship to expected demands on the transportation system. Understanding the amount and type of land use is critical to taking actions to maintain or enhance transportation system operation.
For transportation forecasting, the land use data are stratified into geographical areas called transportation analysis zones (TAZs), which represent the sources of vehicle trip generation. There are approximately 60 RTC TAZs within the Camas TIF study area. As part of the previous Camas TIF update (2003), a detailed land use inventory was conducted for the Camas Urban Growth Area. Information collected from that effort was used to disaggregate RTC "parent" transportation analysis zones (TAZs) into smaller "child" TAZs (see Figure 4). The 60 RTC TAZs were subdivided into about 140 TAZs.

[^3]

The purpose of the disaggregation is to more accurately load traffic onto the street network. Overall, the land uses assumed are consistent with RTC's land use assumptions, which were recently reviewed and updated, regionally.

The disaggregated land use data was reviewed by City staff and refined to reflect local planning efforts. Table 2 summarizes the land uses for the base year model (2005) and the future scenario (2035) within the Camas study area. While these summaries only outline land use in Camas for the purposes of this study, the travel demand forecasts that have been evaluated reflect the regional land use growth throughout the Portland/Vancouver metropolitan area. Table 2 indicates that significant growth is expected in Camas in the coming decades.

Table 2: Camas Land Use Summary

| Land Use | $\mathbf{2 0 0 5}$ | $\mathbf{2 0 3 5}$ | Increase | \% Increase |
| :--- | :--- | :--- | :--- | :--- |
| Households (HH) | 7,021 | 14,124 | 7,103 | $101 \%$ |
| Retail Employees (RET) | 446 | 3,447 | 3,001 | $673 \%$ |
| Other Employees (OTH) | 5,755 | 14,797 | 9,042 | $157 \%$ |

The land use growth listed in Table 2 is different than the land use growth used to develop the 2003 Camas TIF. The 2023 land use used in the 2003 TIF indicated that households would increase by $66 \%$, slightly less than the amount identified for 2035 . The 2035 forecast identifies a significant increase in retail employment growth when compared to the 2023 forecast (a $673 \%$ increase in 2035 versus $32 \%$ increase in 2023). Other employment growth assumed for 2035 is generally comparable to what was assumed for 2023 ( $157 \%$ in 2035 versus $144 \%$ in 2023). The land use developed for the 2035 forecasts includes areas north and east of Lacamas Lake, which were not planned for in the 2023 employment forecast.

At the base year (2005) level of land development, the transportation system generally operates without significant deficiencies in the study area. As land uses are changed in proportion to each other (i.e. there is a significant increase in employment relative to household growth), there will be a shift in the overall operation of the transportation system. Retail land uses generate higher amounts of trips per acre of land than do households and other land uses. The location and design of retail land uses in a community can greatly affect transportation system operation. Additionally, if a community is homogeneous in land use character (i.e. all employment or residential), the transportation system must support significant trips coming to or from the community rather than within the community. Typically, there should be a mix of residential, commercial, and employment type land uses so that some residents may work and shop locally, reducing the need for residents to travel long distances.

## RTC Area Transportation Model

A determination of future traffic system needs in Camas requires the ability to accurately forecast travel demand resulting from estimates of future population and employment for the City. The objective of the transportation planning process is to provide the information necessary for making decisions on when and where improvements should be made to the transportation system to meet travel demand as developed in an urban area travel demand model as part of the Regional Transportation Plan update process. RTC uses VISUM, a computer based program for transportation planning, to process the large amounts of data for the Clark County area. For the Camas TIF Update, the RTC model was used to forecast 2035 travel with substantially more detail added into the Camas area as described previously.
Traffic forecasting can be divided into several distinct but integrated components that represent the logical sequence of travel behavior (Figure 5). These components and their general order in the traffic forecasting process are as follows:

- Trip Generation
- Trip Distribution
- Mode Choice
- Traffic Assignment


## Trip Generation

The trip generation process translates land use quantities (number of dwelling units, retail, and other employment) into vehicle trip ends (number of vehicles entering or leaving a TAZ or subTAZ). The RTC trip generation process is elaborate, entailing detailed trip characteristics for various types of housing, retail employment, non-retail employment, and special activities. Typically, most traffic impact studies rely on the Institute of Transportation Engineers (ITE) research for analysis. ${ }^{11}$ The ITE trip rates are used in implementing TIF fee calculations because they provide a greater link between specific land use and vehicular traffic. The model process is tailored to variations in travel characteristics and activities in the region and is useful for establishing area-wide TIF rates.

Table 3 illustrates the estimated growth in vehicle trips generated within the Camas area during the PM peak period between 2010 and 2035. It indicates that vehicle trips in Camas would grow by approximately 137 percent between 2010 and 2035 if the land develops according to the land use forecasts, with the majority of growth occurring in the north part of the city. This growth is significantly higher than the $95 \%$ growth identified in the 2003 TIF, which is consistent with the change in land use forecasts. Assuming a 25 -year horizon to the 2035 scenario, this represents an annualized growth rate of about 2.9 percent per year.

Table 3: Existing and Future Projected Vehicle Trip Generation (PM Peak Hour)

| Camas UGA | $\mathbf{2 0 0 5}$ | $\mathbf{2 0 3 5}$ | $\mathbf{2 0 3 5 - 2 0 0 5}$ | Change |
| :--- | :--- | :--- | :--- | :--- |
| Trips | 10,313 | 24,483 | 14,170 | $137 \%$ |

[^4]

## Trip Distribution

This step estimates how many trips travel from one zone in the model to any other zone. Distribution is based on land uses, trip purpose, and on factors that relate the likelihood of travel between any two zones to the travel time between zones (including the influences of congestion). In projecting long-range future traffic volumes, it is important to consider potential changes in regional travel patterns. Although the locations and amounts of traffic generation in Camas are essentially a function of future land use in the city, the distribution of trips is influenced by regional growth, particularly in neighboring areas in Clark County, including Vancouver and Washougal. The trip distribution from RTC's regional model was incorporated into the Camas mesoscopic focus-area model to ensure regional consistency.

## Mode Choice

This is the step where it is determined how many trips will be by various modes (single-occupant vehicle, transit, truck, carpool, pedestrian, bicycle, etc.). The 2005 mode splits are incorporated into the base model and adjustments to that mode split are projected for the future scenario, depending on any expected changes in transit or carpool use. These considerations are built into the forecasts used for 2035, consistent with the RTC regional travel demand model.

## Traffic Assignment

Trip assignment involves the determination of the specific travel routes taken by all of the trips within the transportation network. This step was performed using VISUM modeling software. Model inputs included the transportation network (i.e., road and intersection locations and characteristics, as determined from maps and field inventories) and a trip distribution table (determined using methodology described previously in this memorandum). Iterated equilibrium assignment was then performed using estimated travel times along roadways and delays at intersection movements. ${ }^{12}$ The path choice for each trip was based on minimal travel times between locations. Model outputs include traffic volumes on roadway segments and at intersections.

[^5]
## Model Application to Camas

The future base network was developed through coordination with City of Camas staff. The improvements included in the base year model are those projects with secured funding. The base 2035 roadway network included the following projects:

- SR 14 Camas-Washougal Widening and Interchange Improvements:
- Widening of SR 14 from two lanes to four lanes from the end of the West Camas Slough Bridge to Union Street (SR 500)
- Construction of a split-diamond interchange at Union Street and $2^{\text {nd }}$ Street
- Includes four new roundabouts, north and south of SR 14 at Union Street and $2^{\text {nd }}$ Street
- SE $20^{\text {th }}$ Street Improvement from SE Armstrong Road to SE $192^{\text {nd }}$ Avenue - widen existing portion to three lanes with bike lanes and sidewalks and extend to SE $192^{\text {nd }}$ Avenue. ${ }^{13}$


## 2035 Base Traffic Volumes

Intersection turn movements were extracted from the model at key intersections for both the base year 2005 and forecast year 2035 scenarios. These intersection turn movements were not used directly, but the increment of the year 2035 turn movements over the 2005 turn movements was applied (added) to existing (actual 2010) turn movement counts in Camas, since 2010 counts were determined to be comparable to 2005 counts. A post-processing technique following NCHRP 255 methodology was used to refine model travel forecasts to the volume forecasts used for future intersection analysis. The turn movement volumes used for future year intersection analysis can be found in the technical appendix. The traffic volumes developed for the Future 2035 Base are shown in Figure 6.

[^6]| 1. 6 th Ave.Norwood $S$. | 2. 6 th Ave.IIy St. | 3. Division St//6th Ave. | 4. Adams St./Gth Ave. | 5. Dallas St/ISR. 500 (3rd Ave.) | 6a. $s$ R-14 (North Roundabout) | 7. 3 rd Ave./2nd Ave. (4th $S t$. | 8. 3rcd Ave./Crown Rd. |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 9. 6th Ave./SR. 500 (Garield St.) | 10. 14th Ave./SR-500 (Everett St.) | 11. 18th Ave./Division St. | 12. 28 it Ave. Sierra 0 or | 13. 18th Ave./Cascade St | 14. Mclntosh Rd/./rady $K d$. | 15. 16 th Ave. Brady Rd. | 16. Pacific Rim Blvd./Payne Rd. |
| 17. Paciicic Rim Elvd./Parker St. | 18. 38th Ave./Parker St. | 19. Lake Rd//Sierra St. | 20. Lathe Roadss. 50 ( Everert $S t$ ) | 21. 43 rr Ave/SR. 500 (Everett $s t$. .) |  |  |  |
| 22. Leadbetter Rd./SR. 500 (Everertt | 23. Nourse Rd. 15 tht St/283rd Ave. | 24. Lake Rd./Parker St. | 25. Lake Rd.218th Ave. (Payne St.) | 26. 1st St.fFriberg St.-202nd Ave. |  |  |  |
| 27. 13th St.fFiberg St. | 28. Goodwin Rd/Camas Meadows Dr. | 29. Goodwin Rd/Ingle Rd. | 30. 28th St/232nd Ave. |  |  |  |  |
|  |  |  |  | LEGEND <br> (10) - Study Intersection <br> - Stop Sign <br> 8 <br> - Traffic Signal |  |  |  |

## Future Base (2035) Operating Conditions

The 2035 base motor vehicle operating conditions at the study intersections were determined for the evening peak hour based on the 2000 Highway Capacity Manual methodology ${ }^{14}$ for signalized and unsignalized intersections. The conditions include the estimated average delay, level of service (LOS), and volume-to-capacity ( $\mathrm{v} / \mathrm{c}$ ) ratio of the study intersections and are shown in Table $4 .{ }^{15}$

During the evening peak hour, all signalized study intersections operate within jurisdictional standards, with the exception Lake Road/Parker Street and $13^{\text {th }}$ Street/Friberg Street. The Lake Road/Parker Street intersection, while operating at an acceptable level of service, exceeds the City's volume-to-capacity mobility standard by 0.02 , a small amount. However, the intersection at $13^{\text {th }}$ Street/Friberg Street would operate at level of service $F$ and significantly exceed the City's volume-to-capacity standard.

Two of the three all-way-stop controlled intersections would exceed the City's mobility standard, although current tools do not allow correct analysis of the intersection at Pacific Rim Boulevard/Parker Street. The level of service shown, E, reflects an analysis that assumes fewer lanes than currently exist at this intersection, due to analysis limitations. It is likely that this intersection would operate slightly better than what is reported. However, the $16^{\text {th }}$ Avenue/Brady Road intersection does operate poorly, level of service F, and improvements should be considered at that location.

Ten of the unsignalized study intersections deteriorate to a LOS of E or F due to the growth in motor vehicle volumes. These intersections are located on arterial roadways, including $6^{\text {th }}$ Avenue, SR 500/Everett Street, Lake Road, Pacific Rim Boulevard and Goodwin Road/28 ${ }^{\text {th }}$ Street.

[^7]Table 4: Future Base (2035) Evening Peak Hour Intersection Operations

|  | Mobility Standard* |  |  | Level of |
| :--- | :--- | :--- | :--- | :--- |
| Intersection | LOS | V/C | Delay | Service | Capacity | Colume/ |
| :--- |

Signalized Intersections

| Dallas Street/SR-500 (3rd Avenue) | E |  | 13.7 | B | 0.74 |
| :--- | :---: | :---: | :---: | :---: | :---: |
| $3^{\text {rd }}$ Avenue/2 $2^{\text {nd }}$ Avenue-4 ${ }^{\text {th }}$ Street | D | 0.90 | 8.7 | A | 0.55 |
| $3^{\text {rd }}$ Avenue/Crown Road | D | 0.90 | 20.8 | C | 0.69 |
| $38^{\text {th }}$ Avenue/Parker Street | D | 0.90 | 31.1 | C | 0.84 |
| Lake Road/SR-500 (Everett Street) | E |  | 67.1 | E | 1.04 |
| $43^{\text {rd }}$ Avenue/SR-500 (Everett Street) | E |  | 15.0 | B | 0.66 |
| Lake Road/Parker Street | D | 0.90 | 38.7 | D | $\mathbf{0 . 9 2}$ |
| $1^{\text {st }}$ Street/Friberg Street-202 ${ }^{\text {nd }}$ Avenue | D | 0.90 | 15.3 | B | 0.71 |
| $13^{\text {th }}$ Street/Friberg Street | D | 0.90 | 96.7 | F | $\mathbf{1 . 2 2}$ |

All-Way Stop Intersections

| $28^{\text {th }}$ Avenue/Sierra Drive | D | 0.90 | 9.9 | A | 0.37 |
| :--- | :---: | :---: | :---: | :---: | :---: |
| $16^{\text {th }}$ Avenue/Brady Road | D | 0.90 | 88.4 | F | $\mathbf{1 . 2 4}$ |
| Pacific Rim Boulevard/Parker Street** | D | 0.90 | 41.3 | E | $\mathbf{1 . 0 7}$ |

Unsignalized Intersections

| $6^{\text {th }}$ Avenue/Norwood Street | D | 0.90 | $>200.0$ | C/F | $>\mathbf{2 . 0}$ |
| :--- | :---: | :---: | :---: | :---: | :---: |
| $6^{\text {th }}$ Avenue/Ivy Street | D | 0.90 | 182.7 | A/F | 0.55 |
| Division Street/6 |  |  |  |  |  |
| Adams Avenue | D | 0.90 | 24.2 | $\mathrm{~A} / \mathrm{C}$ | 0.51 |
| $6^{\text {th }}$ Avenue/SR-500 (Garfield Street) | E |  | 46.4 | $\mathrm{~A} / \mathrm{E}$ | 0.58 |
| $14^{\text {th }}$ Avenue/SR-500 (Everett Street) | E |  | $>200.0$ | A/F | $>2.0$ |
| $18^{\text {th }}$ Avenue/Division Street | D | 0.90 | 12.7 | $\mathrm{~A} / \mathrm{B}$ | 0.28 |
| $18^{\text {th }}$ Avenue/Cascade Street | D | 0.90 | 16.5 | $\mathrm{~A} / \mathrm{C}$ | 0.14 |
| McIntosh Road/Brady Road | D | 0.90 | 24.6 | $\mathrm{~A} / \mathrm{C}$ | 0.43 |
| Pacific Rim Boulevard/Payne Road | D | 0.90 | 128.4 | A/F | $\mathbf{1 . 0 5}$ |
| Lake Road/Sierra Street | D | 0.90 | 93.9 | B/F | $\mathbf{1 . 0 7}$ |
| Leadbetter Road/SR-500 (Everett Street) | E |  | 88.0 | A/F | $\mathbf{1 . 0 3}$ |
| Nourse Road-15 ${ }^{\text {th }}$ Street/283 ${ }^{\text {rd }}$ Avenue | D | 0.90 | 9.3 | $\mathrm{~A} / \mathrm{A}$ | 0.08 |


| Intersection | $$ |  | Delay | Level of Service | Volume/ Capacity |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Lake Road/218 ${ }^{\text {th }}$ Avenue/Payne Street | D | 0.90 | >200.0 | B/F | >2.0 |
| Goodwin Road/Camas Meadows Drive | D | 0.90 | >200.0 | B/F | >2.0 |
| Goodwin Road/Ingle Road | D | 0.90 | >200.0 | A/F | >2.0 |
| $28^{\text {th }}$ Street/232 ${ }^{\text {nd }}$ Avenue | D | 0.90 | 132.5 | A/F | 0.93 |

Roundabout Intersections

| Union/"C" Street (north) | D |  | 7.9 | A | 0.45 |
| :--- | :---: | :--- | :--- | :--- | :--- |
| Union/11 ${ }^{\text {th }}$ Street (south) | D |  | 4.3 | A | 0.20 |

*Mobility Standard is for City of Camas, except for SR-14, which is WSDOT HSS and SR-500, which is WSDOT Non HSS
**Intersection configuration not allowed in HCM analysis, therefore intersection configuration was modified in Synchro to allow for capacity analysis
Bolded and Shaded indicates mobility standard is not met

Signalized or All Way Stop intersections: All Movements

$$
\begin{aligned}
& \text { LOS }=\text { Level of Service of Intersection } \\
& \text { Delay }=\text { Average Delay of Intersection } \\
& \text { V/C }=\text { Volume-to-Capacity Ratio of Intersection } \\
& \quad \text { (except for AWS where V/C is for worst movement) }
\end{aligned}
$$

Unsignalized intersections: Worst Movement
LOS = Level of Service of Major Street/Minor Street
Delay $=$ Approach Delay of Worst Movement
V/C $=$ Volume-to-Capacity Ratio of Worst Movement

## 2035 Base Link Volumes

In addition to the intersection operation analysis, corridor performance was examined to determine if the growth in traffic volumes exceeded capacity on major routes (arterial and collectors) or if significant volume was added to local or neighborhood routes. Figure 7 shows model link volumes for the 2035 Base condition. Similar to Figure 3, the volume-to-capacity ratios shown do not reflect Highway Capacity Manual analysis, but give a general idea of areas of concern. It shows that a number of key corridors are significantly impacted by growth between 2010 and 2035. Figure 8 shows traffic volume growth between 2005 and 2035. Table 5 lists a summary of the corridor performance findings. The issues identified in Table 5 could potentially be mitigated with access control, roadway widening, parallel route improvements, or new parallel facilities to relieve congestion. Strategies and alternatives for mitigating these concerns will be addressed in Chapter 2.



Table 5: Summary of 2035 Link Volume Capacity Analysis

| Roadway | Limits | Issues |
| :---: | :---: | :---: |
| SR-14 | 192nd to $6^{\text {th }}$ Avenue | - Growth of approximately 2,200 vehicles in the PM peak hour <br> - PM peak volumes approaching capacity in eastbound direction |
| Lake Road | Parker Street to Everett Street/SR 500 | - Growth of approximately 1,000 vehicles in the PM peak hour <br> - PM peak volumes approach or exceed capacity in eastbound direction |
| $13^{\text {th }}$ Street/ Goodwin Road/ $28^{\text {th }}$ Street | $192^{\text {nd }}$ to 242nd | - Growth of 800 to 1,200 vehicles in the PM peak hour <br> - PM peak volumes approach or exceed capacity of the existing roadway |
| SR 500 | Everett to 242nd | - Growth of 900 to 1,200 vehicles in the PM peak hour <br> - PM peak volumes exceed capacity of existing roadway |
| SR 500/ <br> Everett Street | Lake Road to Leadbetter Road | - Growth of 800 to 900 vehicles in the PM peak hour <br> - PM peak volumes approach capacity of existing roadway |
| $242^{\text {nd }}$ Avenue | North of $28^{\text {th }}$ Street | - Growth of approximately 900 to 1,100 vehicles in the PM peak hour <br> - PM peak volumes exceed capacity of existing roadway northbound |
| $1^{\text {st }}$ Street/ <br> Lake Road | $192^{\text {nd }}$ Avenue to Parker Street | - Growth of about 1,500 vehicles in the PM peak hour <br> - PM peak volumes approach capacity of existing roadway |

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## CHAPTER 2: IMPROVEMENTS ALTERNATIVES ANALYSIS

Between 2005 and 2035, the traffic volume within Camas’ Urban Growth Area (UGA) is forecast to grow by 137 percent. Future deficiencies were identified using WSDOT's and the local jurisdiction's thresholds for mobility standards. Improvements to the Camas street system, including intersection improvements, roadway improvements, or new roadways, were considered and a package of recommended improvements was determined. This chapter discusses the recommended roadway improvements, including benefits, costs and related policies.

## Major Roadway Improvements

Several roadway improvements were identified to address the intersection capacity and roadway capacity issues identified in the Existing and Future Baseline Conditions section, previously. Several of the roadway improvements that were tested and recommended were projects originally recommended in the 2003 TIF Update. Other projects include new facilities to serve the North UGA Expansion area, or other improvements determined to meet the latest future forecast demands. Table 6 lists the recommended major roadway improvements and describes their benefits.

Figure 9 shows the volume-to-capacity ratios with the proposed improvements in place.

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Table 6: 2035 PM Peak Hour Mitigated Intersection Level of Service
\(\left.$$
\begin{array}{l|l|l|l}\hline \text { Roadway } & \text { Limits } & \text { Description } & \text { Benefits } \\
\hline \text { Goodwin Road } & 192^{\text {nd }} \text { Avenue to Friberg Street } & \begin{array}{l}\text { An improvement is needed to provide } \\
\text { additional capacity between Vancouver } \\
\text { and Camas. No specific project has been } \\
\text { identified, but could include: } \\
\bullet \quad \text { widening of } 13^{\text {th }} \text { Street }\end{array} & \begin{array}{l}\text { Modeling shows there will be a high } \\
\text { travel demand in the future between }\end{array}
$$ <br>
Vancouver and northern Camas. Either <br>
two three-lane corridors or one five- <br>
lane corridor will be needed to connect <br>

constructing an 18^{th} Street\end{array}\right]\)| 192nd and Goodwin/28th. |
| :--- |

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| Roadway | Limits | Description | Benefits |
| :---: | :---: | :---: | :---: |
| $232^{\text {nd }}$ Street Improvement | Widen and improve $232^{\text {nd }}$ Street between $28^{\text {th }}$ Street and $9^{\text {th }}$ Street | In conjunction with the Ingle Street Extension, eliminates the need for a fivelane section on Goodwin between Ingle and $242^{\text {nd }}$ Avenue | - Provides additional capacity <br> - Provides access to new development area |
| $9^{\text {th }}$ Street <br> Improvement | Widen and improve $9^{\text {th }}$ Street between $232^{\text {nd }}$ Avenue and $242^{\text {nd }}$ Avenue Extension | In conjunction with the Ingle Street Extension and the $232^{\text {nd }}$ Street Improvement, eliminates the need for a five-lane section on Goodwin between Ingle and $242^{\text {nd }}$ Avenue | - Provides additional capacity <br> - Provides access to new development area |
| $242^{\text {nd }}$ Avenue Extension | $28^{\text {th }}$ Street to $14^{\text {th }}$ Street | Construct new high-speed ( 45 mph ) 3lane roadway | - Provide a high mobility roadway connection as an alternative to SR 500 (which would otherwise have high demands in the future) <br> - Provide access to new development |
| New East-West Arterial | $14^{\text {th }}$ Street to SR 500 (Everett Street) | Construct new high-speed 3 lane roadway | - Provide a high-speed, high-capacity roadway connection as an alternative to SR 500 <br> - Provide access to new development |
| NE Everett Street | $35^{\text {th }}$ Avenue to New East-West Arterial | Widen to 3 lanes | - Provide turn lane capacity for adjacent development and growth in through traffic |
| $23^{\text {rd }}$ Street <br> Extension | $43^{\text {rd }}$ Avenue to $283{ }^{\text {rd }}$ Avenue | New 2 lane, minimum access roadway | - Provide access to new development <br> - Provide additional connectivity in the area |

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| Roadway | Limits | Description | Benefits |
| :---: | :---: | :---: | :---: |
| $23^{\text {rd }}$ Street <br> Realignment | $283{ }^{\text {rd }}$ Avenue to $23{ }^{\text {rd }}$ Street | Construct connection between $23^{\text {rd }}$ Street Extension terminus on $283^{\text {rd }}$ Avenue south of $23^{\text {rd }}$ Street to $23^{\text {rd }}$ Street | - Provide a direct connection between the new $23^{\text {rd }}$ Street Extension (at $283^{\text {rd }}$ Avenue) and the existing $23^{\text {rd }}$ Street, providing access east toward Washougal |
| Friberg Street | $1^{\text {st }}$ Street to $13^{\text {th }}$ Street | Widen to 3 lanes | - Provide turn lane capacity for adjacent development and growth in through traffic |
| $38^{\text {th }}$ Avenue <br> Extension | $192^{\text {nd }}$ to Bybee | Construct new 3 lane roadway | - Provide a direct connection to $192^{\text {nd }}$ with adequate capacity rather than a residentially fronted two lane street |
| $38^{\text {th }}$ Avenue <br> (West) | Bybee to Parker | Widen to 3 lanes | - Provide turn lane capacity for adjacent development and growth in through traffic |
| $38^{\text {th }}$ Avenue (East) | Parker to 650 feet west of Dahlia | Widen to 3 lanes | - Provide turn lanes and increased capacity for development |
| Bybee <br> Realignment | $199^{\text {th }}$ Avenue to $20^{\text {th }}$ Street | Realign to meet new signalized intersection | - Current alignment of Bybee would not be access spacing standards between the new signal planned west of $202^{\text {nd }}$ Avenue |

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Notes:

- $13^{\text {th }}$ Street $/ 18^{\text {th }}$ Street Corridor: It is recognized that additional capacity (five lanes total) is needed between NE $192^{\text {nd }}$ Avenue (in Vancouver) and NE Goodwin Road (in Camas). The area between these two points is located within Clark County and, while there are multiple alignment options, there are issues related to each. The most desirable option, in terms of vehicular demand and connectivity, would be a new route along the $18^{\text {th }}$ Street alignment. However, there are known environmental issues with this alignment which would make development of a project very difficult. Another alternative would be to widen NE $13^{\text {th }}$ Street between $192^{\text {nd }}$ Avenue and Goodwin Road, however, this alignment goes through a neighborhood, and would require acquisition of residential property to build a five-lane section. A third alternative would provide two three-lane roadways, however, both environmental and neighborhood issues would need to be addressed. This analysis assumes that some sort of connection is provided (to be determined at a later date), that would provide capacity for the equivalent of a five lane roadway.
- Previous analysis has indicated that a five-lane section would be required along the Goodwin $/ 28^{\text {th }}$ corridor. Current analysis indicates that with the planned improvements in the North UGA area, including a parallel collector route, a three-lane section will work between Ingle and $242^{\text {nd }}$ Avenue. Right-of-way should be reserved for a five-lane section, as ultimately, it may be required.
- In the 2007 Framework Plan, it was recommended that Camas Meadows Drive be realigned to intersect with ${ }^{\text {st }}$ Street/Lake Road at Larkspur/Parker Street. A key purpose of this realignment was to consolidate access and the need for additional traffic signals along $1^{\text {st }}$ Street/Lake Road. Alternatives to this realignment were considered, such as improving the existing Payne Street alignment. However, the Larkspur alignment significantly improves operations at the Lake Road $/ 1^{\text {st }}$ Street/Parker intersection and preserves pedestrian access on all intersection approaches. Pedestrian access may have been at risk on the west approach to the intersection due to the high number of eastbound right turns/northbound left turns that can be reduced by extending Parker north to align with Camas Meadows Drive. Camas Meadows Drive will be improved between Payne Street and Lake Road as a three-lane collector.
- The previous TIF Update recommended improvements to Crown Road. However, current analysis reflects changing development patterns with an increased traffic shed to the north. Current modeling indicates that the current capacity of Crown Road should be adequate to accommodate future growth in Camas.



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## Intersection Improvements

Intersection capacity deficiencies not solved with the major roadway projects were addressed by adding turn lanes, providing signalization or a roundabout where warranted. Eight unsignalized intersections met peak hour signal warrants for the forecast year 2035, as listed in Table 7. Volumes used to determine whether signal warrants were met can be found in the appendix.

Table 7: Future 2035 Signal Warrant Summary at Unsignalized Intersections

| Intersection | Existing Peak Hour Warrant | 2035 Peak Hour Warrant |
| :--- | :---: | :---: |
| $6^{\text {th }}$ Avenue/Norwood Street | No | Yes |
| $6^{\text {th }}$ Avenue/Ivy Street | No | No |
| SR $500 / 14^{\text {th }}$ Avenue | No | No |
| Pacific Rim Boulevard/Payne Rd | No | Yes |
| Lake Road/Sierra Street | No | Yes |
| Leadbetter/SR 500 (Everett) | No | Yes |
| Nourse Road $-15^{\text {th }}$ Street/283 ${ }^{\text {rd }}$ | No | No |
| $242^{\text {nd }} / 28^{\text {th }}$ Street | No | Yes* |
| Lake Road/218 $^{\text {th }} /$ Payne | No | No |
| Goodwin Road/Camas Meadows $_{\text {Goodwin/Ingle }}^{28^{\text {th }} / 232^{\text {nd }} \text { Avenue }}$ | No | Yes |
| Brady $/ 16^{\text {th }}$ | No | Yes |
| Parker/Pacific Rim | No | No |

* No existing count available, future volume estimated based on model volumes

None of these locations met signal warrants under existing conditions. Traffic pattern changes are planned at one of the intersections (Leadbetter/SR 500) that would mitigate the need for a traffic signal at this location. Two additional locations were identified as potential roundabout locations (Everett Street/SR 500/Lake Road and 6th Avenue/Norwood Street), and are addressed below. The recommended TIF signal improvements are at the following nine locations:

- $6^{\text {th }}$ Avenue/Norwood Street
- Pacific Rim Boulevard/SE Payne Road
- Lake Road/Sierra Street
- Goodwin Road/Camas Meadows Drive
- Goodwin Road/Ingle Street
- Brady Road $/ 16^{\text {th }}$ Street
- Parker Street/Pacific Rim Boulevard
- $242^{\text {nd }} / 28^{\text {th }}$ Street


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## Roundabouts

Roundabouts are being considered as alternatives to improvements at Everett Street/SR 500/Lake Road and $6^{\text {th }}$ Avenue/Norwood Street for different reasons. Each is discussed below:

- Everett Street/SR 500/Lake Road: This intersection is currently signalized and will not meet operational standards in 2035 with its existing configuration. Due to a bridge immediately north of the intersection, the addition of an additional southbound lane (which would address the capacity deficiency) would be extremely costly, potentially more than $\$ 5$ million. There is some undeveloped land, however, to the east of the intersection that may be suitable for reconfiguration with roundabout control. Coincidentally, the land is owned by the City's Parks Department. Due to the relatively balanced traffic volumes approaching the intersection, the availability of land nearby and the constraint of the bridge to the north, the potential for a roundabout at this location was evaluated. Based on the projected 2035 volumes, a partial multi-lane roundabout at this location would operate at level of service B, well within the acceptable standards for both the City of Camas and the Washington State Department of Transportation. Since this intersection is located along SR 500, input and cooperation from WSDOT will be essential.

Recommendation: A roundabout would function well at this location. Both turn lane and roundabout improvement options should be considered as design options. Include the lower cost of the two options for TIF funding.

- $6^{\text {th }}$ Avenue/Norwood Street: This intersection is currently unsignalized. The level of service for side street traffic is poor (LOS F) today and is projected to decline even further in the future. While traffic signal warrants would be met at this location in the future, a traffic signal at this location may be disruptive to the large volume of traffic traveling east and west through the intersection. A roundabout would allow continuous flow for these heavy movements, while allowing side street traffic a much improved level of service. An additional benefit of a roundabout at this location is its potential to slow traffic coming off of SR 14 an entering the City of Camas. It could be a natural transition from the high speeds on the state highway to slower speeds in town. This roundabout would incorporate ramps to and from SR 14, so input and cooperation from WSDOT is essential. The cost of a roundabout at this location would be substantial, however, due to grade issues, potentially in the multi-million dollar range. A traffic signal would cost substantially less, so a signal will be recommended at this location as part of this TIF Update.


## Recommendation: Install a Traffic Signal rather than a Roundabout at this location due to cost.

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## 2035 Improved Operational Analysis

Intersection capacity analysis was conducted at each of the study intersections, including the recommended major roadway improvements and intersection projects. Table 8 lists the results of the analysis. Each of the study intersections operates at a LOS of D and v/c ratio of 0.90 or better, with the exception of $6^{\text {th }} /$ Ivy, $6^{\text {th }} /$ Garfield, Lake/Payne and $28^{\text {th }} / 232$ nd. Each of these intersections operate at a LOS E or F for the minor street left turn. Signal warrants are not met at any of these locations and volume-to-capacity ratios for affected movements are relatively low (less than 0.90), therefore no further improvements are recommended. These locations should be monitored to determine if signalization does become warranted at some time in the future with local development.

Table 8: 2035 PM Peak Hour Mitigated Intersection Operations

| Intersection | Mobility Standard* <br> LOS V/C |  | Delay | Level of Service | Volume/ Capacity |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Signalized Intersections |  |  |  |  |  |
| Dallas Street/SR-500 (3rd Avenue) | E |  | 13.9 | B | 0.74 |
| $3{ }^{\text {rd }}$ Avenue/2 ${ }^{\text {nd }}$ Avenue-4 $4^{\text {th }}$ Street | D | 0.90 | 8.7 | A | 0.54 |
| $3{ }^{\text {rd }}$ Avenue/Crown Road | D | 0.90 | 20.8 | C | 0.69 |
| $38^{\text {th }}$ Avenue/Parker Street | D | 0.90 | 33.8 | C | 0.85 |
| $43^{\text {rd }}$ Avenue/SR-500 (Everett Street) | E |  | 13.5 | B | 0.60 |
| Lake Road/Parker Street | D | 0.90 | 53.1 | D | 0.90 |
| $1{ }^{\text {st }}$ Street/Friberg Street-202 ${ }^{\text {nd }}$ Avenue | D | 0.90 | 21.3 | C | 0.77 |
| $13^{\text {th }}$ Street/Friberg Street | D | 0.90 | 26.4 | C | 0.84 |
| New Signals |  |  |  |  |  |
| $6^{\text {th }}$ Avenue/Norwood Street | D | 0.90 | 25.8 | C | 0.63 |
| $16^{\text {th }}$ Avenue/Brady Road | D | 0.90 | 15.7 | B | 0.76 |
| Pacific Rim Boulevard/Parker Street | D | 0.90 | 20.1 | C | 0.48 |
| Pacific Rim Boulevard/Payne Road | D | 0.90 | 14.5 | B | 0.59 |
| Lake Road/Sierra Street | D | 0.90 | 25.9 | C | 0.77 |
| Goodwin Road/Camas Meadows Drive | D | 0.90 | 29.6 | C | 0.90 |
| Goodwin Road/Ingle Road | D | 0.90 | 31.7 | C | 0.73 |
| All-Way Stop Intersections |  |  |  |  |  |
| $28^{\text {th }}$ Avenue/Sierra Drive | D | 0.90 | 11.4 | B | 0.43 |

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| Intersection | Mobility Standard*$\text { LOS } \quad \text { V/C }$ |  | Delay | Level of Service | Volume/ Capacity |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Unsignalized Intersections |  |  |  |  |  |
| $6{ }^{\text {th }}$ Avenue/Ivy Street | D | 0.90 | 84.1 | A/F | 0.32 |
| Division Street/6 ${ }^{\text {th }}$ Avenue | D | 0.90 | 28.4 | A/D | 0.66 |
| Adams Street/6 dr $^{\text {th }}$ Avenue** | D | 0.90 | 19.3 | A/C | 0.45 |
| $6^{\text {th }}$ Avenue/SR-500 (Garfield Street) | E |  | 47.8 | A/E | 0.58 |
| $14^{\text {th }}$ Avenue/SR-500 (Everett Street) | E | Not an intersection, as proposed |  |  |  |
| $18^{\text {th }}$ Avenue/Division Street | D | 0.90 | 14.5 | A/B | 0.32 |
| $18^{\text {th }}$ Avenue/Cascade Street | D | 0.90 | 16.4 | A/C | 0.02 |
| McIntosh Road/Brady Road | D | 0.90 | 33.7 | A/D | 0.53 |
| Leadbetter Road/SR-500 (Everett Street) | E | Right-in/Right-out only, as proposed |  |  |  |
| Nourse Road-15 ${ }^{\text {th }}$ Street/283 ${ }^{\text {rd }}$ Avenue | D | 0.90 | 14.5 | A/B | 0.28 |
| Lake/Payne | D | 0.90 | 52.6 | B/F | 0.81 |
| $28^{\text {th }}$ Street/232 ${ }^{\text {nd }}$ Avenue | D | 0.90 | 62.4 | A/F | 0.56 |
| Roundabout Intersections |  |  |  |  |  |
| Lake Road/SR-500 (Everett Street) | E |  | 22.0 | C | 0.92 |
| Union/"C" Street (north) | E |  | 16.1 | B | 0.59 |
| Union/11 ${ }^{\text {th }}$ Street (south) | E |  | 13.0 | B | 0.16 |
| Signalized or All Way Stop intersections: All Movements <br> LOS = Level of Service of Intersection <br> Delay = Average Delay of Intersection <br> V/C = Volume-to-Capacity Ratio of Intersection <br> (except for AWS where V/C is for worst movement) |  | Unsignalized intersections: Worst Movement <br> LOS = Level of Service of Major Street/Minor St <br> Delay = Approach Delay of Worst Movement <br> V/C = Volume-to-Capacity Ratio of Worst Movement <br> Roundabout intersections: Worst Movement |  |  |  |

## DKS Associates

## Recommended TIF Improvements

The improvements identified to mitigate future growth impacts to the transportation system are listed in Table 9 and shown in Figure 10. Cost estimates were completed for each project, which include all project related costs, with potential right-of-way costs shown separately. The projects are not listed in order of priority. Prioritization should occur in coordination with the CIP process. All TIF improvements include sidewalks for pedestrians, bike lanes for bicyclists, and transit facilities for buses and park-and-riders. This improvement program meets the TIF requirement to establish a nexus between capacity needs and future land use.

The updated TIF project listing, while extensive, is not intended to represent the comprehensive listing of all transportation improvement in Camas. Other transportation improvements (turn lanes, street modernization, traffic calming, bicycle, pedestrian, and transit improvements beyond those programmed) may be built as part of fronting development improvements, SEPA required mitigation, or other processes.

## Cost Estimates

Cost estimates were developed for each improvement based upon 2011 dollars. Past construction information in the region was utilized as a basis for updates to the unit costs from the previous TIF Update study (2003). Each roadway project was estimated, including the total project cost of the roadway improvement including engineering, construction, and landscaping. In addition, the TIF eligible portion is listed as well. The TIF eligible portion is described later, but generally consists of curb-to-curb plus storm sewer costs. Where projects go outside of the Camas UGA, TIF eligible project costs include only the expected Camas share, based on growth. Potential right-of-way costs are shown separately.

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Table 9: Camas UGA TIF Improvements

| Element | Improvement Project | Improvement | Total Construction Project Cost (millions) | TIF Eligible Cost (millions) |
| :---: | :---: | :---: | :---: | :---: |
| A | Goodwin Road <br> (Lacamas Creek to Ingle Road) | Widen from 2 to5 lanes between Friberg Street and Ingle Road | \$4.9 | \$4.5 |
| B | Goodwin Road (Ingle Road to 232 ${ }^{\text {nd }}$ Avenue | Widen from 2 lanes to 3 lanes between Ingle Road and $232{ }^{\text {nd }}$ Avenue | \$6.4 | \$4.5 |
| C | Goodwin Road ( $232^{\text {nd }}$ Avenue to $242^{\text {nd }}$ Avenue | Widen from 2 lanes to 3 lanes between $232^{\text {nd }}$ Avenue and 242 ${ }^{\text {nd }}$ Avenue | \$3.2 | \$0.8 |
| D | New East-West Collector (extend Ingle Road to $232^{\text {nd }}$ Avenue) | Extend Ingle Road south of Goodwin/28 $8^{\text {th }}$ as a 3 lane road to $232^{\text {nd }}$ Avenue | \$7.4 | \$5.1 |
| E | Improve $232^{\text {nd }}$ Avenue | Improve $232^{\text {nd }}$ Avenue to 3 lane Collector from NE $28^{\text {th }}$ Street to $9^{\text {th }}$ Street. Includes 2 new roundabouts at intersection with new East-West Collector and at ${ }^{9 \text { th }}$ Street | \$7.8 | \$4.7 |
| F | Improve/Extend 9 ${ }^{\text {th }}$ Street | Improve $9^{\text {th }}$ Street to 3 lane collector from $232^{\text {nd }}$ Avenue to existing terminus and extend to new $242^{\text {nd }}$ Avenue Extension | \$3.7 | \$2.9 |
| G | Extend $242^{\text {nd }}$ Avenue south to $9^{\text {th }}$ Street | Extend and widen to 3 lanes between $28^{\text {th }}$ to $9^{\text {th }}$ Street | \$9.5 | \$4.5 |
| H | New East-West Arterial | New 3 lane roadway between $9^{\text {th }}$ Street and SR 500/Everett Street | \$11.5 | \$9.0 |
| I | Widen NE Everett Street | Widen from 2 lanes to 3 lanes between $35^{\text {th }}$ Avenue and the new EastWest Arterial | \$4.7 | \$3.6 |
| S | $192{ }^{\text {nd }}$-Goodwin Connector | Camas share (39\%) of potential connection between $192^{\text {nd }}$ and Goodwin. Specific project and alignment to be determined. (North proportionate cost only) | \$2.8 | \$0.9 |
| North Roadway Projects |  |  | \$61.9 | \$40.5 |

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| Element | Improvement Project | Improvement | Total Construction Project Cost (millions) | TIF Eligible Cost (millions) |
| :---: | :---: | :---: | :---: | :---: |
| J | Woodburn Drive (Greg Reservoir area) | New 2 lane roadway between $15^{\text {th }}$ Street and $283{ }^{\text {rd }}$ Avenue. | \$5.3 | \$3.8 |
| K | $23^{\text {rd }}$ Street Realignment | Realign $23^{\text {rd }}$ Street east of $283^{\text {rd }}$ Avenue to intersect with new East-West Collector | \$0.6 | \$0.5 |
| L | Friberg ( $1^{\text {st }}$ Street to $13^{\text {th }}$ Street) | Widen from 2 lanes to 3 lanes between $1^{\text {st }}$ Street and $13^{\text {th }}$ Street | \$5.0 | \$3.9 |
| M | Extend Camas Meadows <br> Drive | Extend Camas Meadows Drive from Payne Street to Lake Road as a three lane collector, includes signal modification at Lake/1 $1^{\text {st }} /$ Parker | \$3.8 | \$2.9 |
| N | $38^{\text {th }}$ Avenue Extension | New 3 lane roadway between 650 feet east of Bybee and 500 feet east of $192^{\text {nd }}$ | \$2.7 | \$2.0 |
| O | Bybee Realignment | Realign Bybee between NW $199^{\text {th }}$ and SE $20{ }^{\text {th }}$ | \$1.2 | \$1.0 |
| P | Widen $38^{\text {th }}$ Avenue (West) (650 feet east of Bybee to Parker) | Widen from 2 lanes to 3 lanes between 650 feet east of Bybee and Parker Street | \$4.7 | \$3.7 |
| Q | Widen $38^{\text {th }}$ Avenue (East) (Parker Street to 800 feet west of Dahlia) | Widen from 2 lanes to 3 lanes between Parker Street and Astor Street | \$2.9 | \$2.2 |
| R | Goodwin Road (Friberg Road to Lacamas Creek) | Widen from 2 to5 lanes between Friberg Street and Ingle Road and Lacamas Creek | \$5.9 | \$4.8 |
| S | $192^{\text {nd }}$-Goodwin Connector | Camas share ( $39 \%$ ) of potential connection between $192^{\text {nd }}$ and Goodwin. Specific project and alignment to be determined. <br> (South proportionate cost only) | \$4.0 | \$1.3 |
| South Roadway Projects |  |  | \$36.1 | \$26.1 |
| Total Roadway Projects (North + South) |  |  | \$98.0 | \$66.6 |

## DKS Associates

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| Element | Improvement Project | Improvement | Total Construction Project Cost (millions) | TIF Eligible Cost (millions) |
| :---: | :---: | :---: | :---: | :---: |
| 1 | $242{ }^{\text {nd }}$ Avenue/Goodwin/28th | Install a traffic signal. Add SB left turn lane. | \$0.5 | \$0.14 |
| 2 | Ingle Road $/ 28^{\text {th }}$ Street | Install a traffic signal. | \$0.25 | \$0.25 |
| 3 | $232{ }^{\text {nd }}$ Avenue/ $22^{\text {nd }}$ Street | Install roundabout | \$0.5 | \$0.27 |
| 4 | $232^{\text {nd }}$ Avenue/ $/{ }^{\text {th }}$ Street | Install roundabout | \$0.5 | \$0.50 |
| 5 | SR 500/New Road ( $242{ }^{\text {nd }}$ Avenue Extension) | Install traffic signal | \$0.25 | \$0.25 |
| 6 | SR 500/Leadbetter | Install median, converting intersection to right-in/right-out only access | \$0.05 | \$0.05 |
| North Intersection Projects |  |  | \$2.05 | \$1.45 |
| 9 | Camas Meadows Drive/Goodwin Road | Install traffic signal. | \$0.25 | \$0.25 |
| 10 | Lake Road/Sierra Street | Install traffic signal. | \$0.25 | \$0.25 |
| 11 | Lake Road/Everett Street/ SR 500 | Install roundabout with two approach lanes on west, east and south legs, and one approach lane on north leg due to bridge limitations to north. | \$2.0 | \$2.0 |
| 12 | $14^{\text {th }} /$ Everett/SR 500 | Install barrier restricting access to intersection from south and west approaches. | \$0.05 | \$0.05 |
| 13 | $6^{\text {th }}$ Avenue/Norwood Street | Install traffic signal | \$0.25 | \$0.25 |
| 14 | Payne Road/ <br> Pacific Rim Boulevard | Install Traffic Signal | \$0.25 | \$0.25 |
| 15 | Brady Road/16 ${ }^{\text {th }}$ Avenue | Install Traffic Signal | \$0.25 | \$0.25 |
| 16 | Parker Street/ <br> Pacific Rim Boulevard | Install Traffic Signal | \$0.25 | \$0.25 |
| South Intersection Projects |  |  | \$3.55 | \$3.55 |
|  |  | Total Cost of Intersection Improvement Projects | \$5.6 | \$5.0 |
|  |  | Right-of-Way Costs | \$32.3 | \$8.0 |
|  |  | Total TIF Improvement Cost (Roadway + Intersection) | \$135.9 | \$79.6 |



## DKS Associates

## TIF Cost Comparison

The cost of transportation improvements in the current TIF Update is expected to be about $\$ 100$ million in today's dollars, not including right-of-way costs. This reflects anticipated growth related needs through 2035. Previous project improvement costs were developed as part of three different projects:

- Camas TIF Update (2003): about $\$ 27$ million in 2003 (plus right-of-way costs)
- North UGA Transportation Improvement Framework Plan: about \$119 million in 2007 (plus right-of-way costs)
- Greg Reservoir Improvements: about \$3.94million in 2005 (includes only TIF eligible costs, right-of-way costs would be additional)

The current TIF Update would reflect a combination of the three as well as any new improvements identified. While construction costs increased since 2003, they have also come down, particularly after 2008. Cost estimates across all time periods listed above would be relatively comparable. While the current TIF update costs appear to be lower than the three plans previously developed, it should be considered that some projects previously identified have already been constructed or are underway (previous cost estimate shown):

- $1^{\text {st }}$ Street/Lake Road - constructed ( $\sim \$ 3.0$ million)
- Leadbetter Road - constructed ( $\sim 3.8$ million)
- SR 14 - project underway ( $\sim 1.8$ million contribution)

Other projects are not included, for a variety of reasons:

- $18^{\text {th }}$ Street Corridor $-192^{\text {nd }}$ to Goodwin: It is recognized that some sort of improvement is necessary to provide additional capacity between $192^{\text {nd }}$ and Goodwin. This area is outside of the Camas UGA and there are multiple options for providing the needed capacity. It could be a new corridor along the $18^{\text {th }}$ Street alignment, widening of $13^{\text {th }}$ Street, or some combination of the two. ( $\sim 7.8$ million)
- $6^{\text {th }}$ Avenue restriping/Road Diet: ( $\$ .71$ million)
- $38^{\text {th }}$ Avenue Extension (Astor to Sierra): ( $\$ 2.5$ million)
- Extend Camas Meadows Drive: ( $\sim 1.8$ million)
- Widen and realign Camas Meadows Drive to $1^{\text {st }} / L a k e / P a r k e r: ~(\sim \$ 4.5$ million)
- Widen Crown Road: ( $\sim \$ 14.2$ million)

Other projects were modified:

- NE $28^{\text {th }}$ Street between $232^{\text {nd }}$ and $242^{\text {nd }}$ (reduced from 5-lane section to 3-lane section) ( $\sim \$ 5.9$ million before vs. $\sim \$ 3.7$ million for the current project)
- $38^{\text {th }}$ Avenue Widening (Parker to Astor): ( $\$ 3.1$ million) - the scope of this project was reduced to include the area between Parker and approximately 800 feet west of Dahlia Street, reducing the overall cost slightly.


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## CHAPTER 3: TIF STRUCTURE

The current traffic impact fee calculation methodology has been utilized since 2003. The basis of the calculation is the assessment of PM peak hour vehicle trips from the Institute of Transportation Engineer's Trip Generation: An ITE Informational Report and a cost rate applied to each trip-end on a citywide basis. Chapter 5 of the previous TIF study provides background into the basis of the TIF. The following sections summarize the key components of the staff's recommended proposed TIF update:

- TIF will be collected based on PM peak hour trip generation rates
- Two TIF districts will be formed (see Figure 11) with project costs allocated either to the North district or the South district, with the exception of the $192^{\text {nd }} /$ Goodwin connector project, which would be allocated between the districts proportionate to their use of the connector, based on growth.
- TIF will fund curb-to-curb plus storm sewer costs
- TIF will fund right-of-way outside the UGA proportionate to the expected Camas share of each project
- TIF will fund $20 \%$ of right-of-way inside the UGA
- TIF costs will be indexed at $3.9 \%$ per year, with new rates taking effect the first of each year

Table 10 summarizes staff's recommendation and the anticipated TIF fee associated with this recommendation, along with adjustments that would be made based upon a $60 \%$ reduction factor (as described previously).

Table 10: Staff Recommended TIF Fee

| TIF Fee Summary | North | South |
| :--- | ---: | ---: |
| Curb-to-Curb+Storm+ROW* | $\$ 10,619$ | $\$ 4,042$ |
| 60\% reduction Factor | $-\$ 4,248$ | $-\$ 1,617$ |
| 2011 Net Rate | $\mathbf{6 6 , 3 7 1}$ | $\mathbf{\$ 2 , 4 2 5}$ |
| 2012 Net Rate | $\$ 6,620$ | $\$ 2,520$ |
| 2013Net Rate | $\$ 6,878$ | $\$ 2,618$ |
| 2014 Net Rate | $\$ 7,146$ | $\$ 2,720$ |
| 2015 Net Rate | $\$ 7,425$ | $\$ 2,826$ |
| 2016 Net Rate | $\$ 7,715$ | $\$ 2,936$ |
| 2017 Net Rate | $\$ 8,015$ | $\$ 3,051$ |
| 2018 Net Rate | $\$ 8,328$ | $\$ 3,170$ |
| 2019 Net Rate | $\$ 8,653$ | $\$ 3,294$ |

* Includes ROW outside the UGA $+20 \%$ of ROW inside UGA



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## Recommended TIF Structure Summary

Table 11 summarizes the recommended TIF structure.
Table 11: TIF Structure Summary

| TIF Element | Basis |
| :--- | :--- |
| Land Use <br> Categories | Latest Edition of ITE Trip Generation: An ITE Informational Report |
| Trip Generation | Based upon highest one hour trip rate in the 4 PM to 6 PM time period from ITE <br> Trip Generation: An ITE Informational Report |
| Pass-by and <br> Diverted Linked <br> Trip Adjustment | Reductions allowed for pass-by and diverted linked trips for land use codes as <br> documented in the Trip Generation Handbook, or with data approved by the City <br> Engineer |
| Trip Length | Not Included |
| Area of Coverage | 2 Districts (North District and South District) per Figure 11 |
| Point of TIF <br> Collection | Building Permit issuance or as otherwise provided by code |
| TIF Project <br> Priorities | Set by the City of Camas Adopted CFP, 6-year street plan, and annual budget. |
| Inflation | Use Washington State Department of Transportation Construction Cost Indices to <br> index TIF as noted in the TIF Rates Alternatives Analysis Memo (see appendix). |
| Changes in Trip <br> Rates | Where a use is not addressed in the ITE Trip Generation: An ITE Report, the <br> applicant may be requested to provide research counts of comparable sites, per ITE <br> recommended practice |
| Credits | Only for construction projects listed in the TIF. Credits not issued unless work is <br> completed. Credits will be issued based on the cost estimate of the TIF project, the <br> reduction factor, and the TIF rate multiplier. When projects are partially completed, <br> a prorated credit based on percentage of the TIF cost estimate will be applied. |
| Exemptions | Per Camas Municipal Code. |
| Appeals | Approved or denied by the Board of Adjustment. |

## Supporting Policy Recommendations

## Reimbursement Costs

Washington state law allows for the collection of some reimbursement costs within the TIF. A bond has been taken out against the TIF to build the previously completed Parker Street and Lake Road projects. The current balance of the bond debt is $\$ 3,077,193.67$.

Since the bond was taken out with the intent of paying it back using TIF funds, this amount is included in the updated TIF.

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## Late Comer's Agreements

Where projects are undertaken and the timing of development does not match with the need for the improvement, the City may undertake the full street improvement and assess late comers agreements with fronting property owners that, at the time, do not participate in funding their share of the fronting improvements costs. At the time this fronting land eventually develops, the City would collect the equivalent balance of roadway improvement costs through the late comer's agreement. This would assure that the TIF is financially solvent and that the fair cost of the street improvements is allocated appropriately to fronting properties - even though at the time of improvement some of the properties are not ready to develop.

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## APPENDICES

Appendix A: Traffic Counts
Appendix B: Existing Level of Service Analysis
Appendix C: Existing Signal Warrants
Appendix D: Focus-Area Mesoscopic Forecasting Methodology Memo
Appendix E: Land Use Assumptions (by TAZ)
Appendix F: Future (2035) Level of Service Analysis
Appendix G: TIF Rate Alternatives Analysis Memo
Appendix H: Future (2035) Improved Level of Service Analysis
Appendix I: Future (2035) Signal Warrants
Appendix J: Cost Estimates

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TRANSPORTATION SOLUTIONS

APPENDIX A TRAFFIC COUNTS

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TRANSPORTATION SOLUTIONS

APPENDIX B
EXISTING INTERSECTION LEVEL OF SERVICE ANALYSIS

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APPENDIX C
EXISTING SIGNAL WARRANTS

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APPENDIX F FUTURE (2035)

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APPENDIX G
FUTURE (2035) SIGNAL WARRANTS

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APPENDIX H
TIF RATE ALTERNATIVES ANALYSIS MEMO

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## APPENDIX I

 FUTURE (2035) IMPROVED INTERSECTION LEVEL OF SERVICE ANALYSIS
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APPENDIX J


[^0]:    ${ }^{1}$ Highways of Statewide Significance, WSDOT, http://www.wsdot.wa.gov/planning/HSS/Default.htm
    ${ }^{2}$ City of Camas Transportation Comprehensive Plan, Transportation Designations, December 2007.
    ${ }^{3}$ Historical Count Data obtained from the City of Camas.

[^1]:    ${ }^{4}$ Level of Service Standards for Washington State Highways, WSDOT, January 1, 2010.
    ${ }^{5}$ Clark County Code, Section 40.350.020, Transportation Concurrency Management System.
    ${ }^{6}$ City of Camas Comprehensive Plan, Transportation Element, Policy TR-20, March 2004.

[^2]:    ${ }^{9}$ Manual on Uniform Traffic Control Devices 2003 Ed., Federal Highway Administration, November 2004.

[^3]:    ${ }^{10}$ Memorandum from DKS Associates to Mark Harrington, RTC, May 20, 2011. Focus-Area Mesoscopic Forecasting Methodology.

[^4]:    ${ }^{11}$ Trip Generation: An ITE Informational Report, 8 至 Edition, Institute of Transportation Engineers, 2008.

[^5]:    ${ }^{12}$ Roadway travel times were calculated based on distance and travel speed. Intersection movement delays were calculated using Highway Capacity Manual (HCM) methodology for signalized and unsignalized intersections. Detailed lane geometry, traffic control, roadway cross-sections, and roadway travel speed information is required for model accuracy.

[^6]:    ${ }^{13}$ Loan payback remnant may be required in new TIF calculation.

[^7]:    142000 Highway Capacity Manual, Transportation Research Board, Washington DC, 2000.
    ${ }^{15}$ Detailed intersection analysis worksheets are attached in the technical appendix.

